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## SONISCOPE INVESTIGATION OF ELMENDORF AFB HOSPITAL ANCHORAGE, ALASKA

Ъу

H. T. Thornton, Jr.

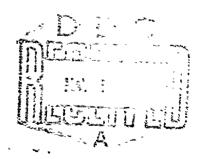


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62

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#### FOREWORD

The soniscope investigation of concrete at Elmendorf AFB Hospital was authorized by a series of telephone calls between Mr. Harold Stuart, U. 3. Army Engineer Division, North Pacific (NPD), Mr. R. E. Holmes, NPD, Mr. W. M. Knopp, U. S. Army Engineer District, Alaska, and Mr. T. B. Kennedy and Mr. Bryant Mather, Concrete Division, U. S. Army Engineer Waterways Experiment Station (WES) on 21, 25, and 26 August 1964, and confirmed by letter from the Division Engineer, NPD, dated 27 August 1964. The investigation was conducted during the period 28 August-16 September 1964 by Messrs. H. T. Thornton, Jr., and J. K. Allgood, Jr., WES, with the assistance of Mr. W. O. Thompson, Project Engineer, Alaska District, and Mr. Rodger Frank, Fort Worth District.

This report was prepared by Mr. Thornton under the supervision of Messrs. T. B. Kennedy, B. Mather, and E. E. McCcy, Jr., all of the Concrete Division, WES.

Directors of the WES during the conduct of this investigation and the preparation and publication of this report were Col. Alex G. Sutton, Jr., CE, and Col. John R. Oswalt, Jr., CE. Technical Director was Mr. J. B. Tiffany.

### CONTENTS

Pag	ge
REWORD	ii
MMARY	ii
ART I: INTRODUCTION	1
The Problem	1
RT II: TEST EQJIFMENT AND PROCEDURES	3
Equipment	3 3
RT III: TESTS AND RESULTS	Ļ
	4 L3
RT IV: CONCLUSIONS	15
ELECTED BIBLIOGRAPHY	16
BLES 1-36	

#### SUMMARY

The hospital building at Elmendorf AFB, Anchorage, Alaska, was damaged by the earthquake of 27 March 1964. Developments occurred during the progress of work designed to repair the damaged concrete which led to a request for a soniscope investigation. The U. S. Army Engineer Waterways Experiment Station (WES) furnished a soniscope crew to make velocity measurements or concrete of suspected inferior quality in areas designated as most critical from a structural standpoint.

On the hospital building proper, 644 sonic velocity measurements were made. The measurements were made through various walls, columns, and beams to determine the representative velocity of the concrete or to find any cracked, shattered, or otherwise inferior concrete present at points where measurements were taken. Using appropriate combinations of the same points, diagonal measurements were also taken through the concrete to locate inferior concrete, particularly horizontal cracking, not detected by the straight-through measurements. In addition to locating inferior concrete, the pulse velocity measurements provided a good indication of the effectiveness of repair by epoxy grouting. To demonstrate this fact, one wail, used as a test panel, was marked with an array of points at which readings were taken before and after grouting with epoxy. Also in this connection, fifteen readings were taken on three 4-in.-diameter, epoxy-repaired cores, and two readings were taken on two 4-in.-diameter, undamaged cores, later broken in compression. Tensilesplitting tests were performed on 2-in.-diameter, epoxy-repaired cores.

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Representative velocities of good concrete ranged between 13,000 and 15,000 fps. Some low velocities were exhibited in all sections tested, but low velocities were far more prevalent in ungrouted sections. Very significant increases in ultrasonic pulse velocities were obtained where both faces of an area of concrete were sealed and the section pumped to refusal with epoxy grout progressing from low to higher elevations.

Results of tests indicate that: the compressive strength of concrete in the hospital building probably ranges between .000 and 5000 psi; the epoxy-repaired concrete has a tensile-splitting strength as high as the original concrete; there are areas within the structure where epoxy repair is not complete and further penetration may not be possible unless the area is sealed and pumped from both sides.

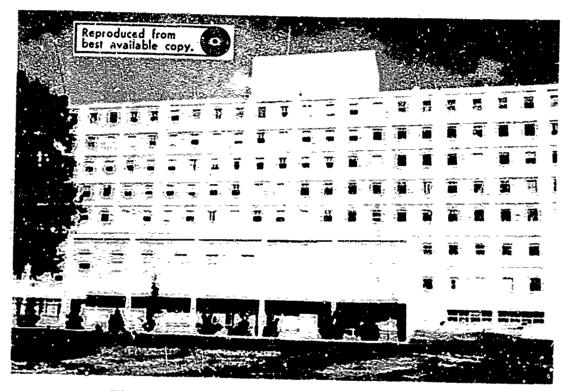


Fig. 1. South sile of Elmendorf AFB Hospital

# SONISCOPE INVESTIGATION OF EIMENDORF AFB HOSPITAL ANCHORACE, ALASKA

#### PART I: INTRODUCTION

### The Problem

1. The hospital building at Elmendorf AFB, Anchorage, Alaska, was one of the many structures lamaged by the earthquake which occurred on 27 March 1964 (fig. 1). During the progress of work designed to repair the concrete by grouting with epoxy resin, it became apparent that the work was not completely successful. Complete filling could be obtained in the fine cracks, but the wider cracks were not always completely filled. Cores drilled from cracked areas showed compressive strengths ranging from 100 to 5000 psi. These developments led to a request for a soniscope investigation. It was thought that a well-planned series of pulse velocity readings would permit an intelligent estimate of the condition of the structure and of the success of the grouting operations which were in progress.

### Purpose and Scope of Study

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- 2. The U. S. Army Engineer Waterways Experiment Station (WES) furnished a soniscope and crew to make velocity measurements on concrete of suspected inferior quality in the hospital to locate any concrete damaged by cracking or shattering that might not be evident from a visual inspection and to obtain an indication of the success of repair by epoxy grouting.
- 3. The investigation was concentrated in areas designated as most critical from a structural standpoint, namely, main shear walls, columns, beams, and column-beam connections. On the hospital building proper, 644 readings were taken. Fifteen readings were taken on three 4-in.-diameter, epoxy-repaired cores, and two readings were taken on two 4-in.-diameter cores later broken in compression. One wall, used as a test panel, was marked with an array of check points at which readings

were taken before and after grouting. These 4-in.-diameter cores were also tested to determine what effect epoxy repair had on ultrasonic pulse velocity in concrete. Tensile-splitting tests were performed on 2-in.-diameter, congrete. Tensile-splitting tests were performed on 2-in.-diameter, congrete cores to compare the strength of the epoxy bond in cracked concrete with that of the undamaged concrete.

#### PART II: TEST EQUIPMENT AND PROCEDURES

### Equipment

4. The schiscope equipment used was similar to that described in Corps of Engineers test method CRD-C 51-57. The schiscope is an instrument that transmits pulses of ultrasonic waves through a material, and electronically measures the time of travel from the transmitter to a receiver while each is held against the surface of the material a known distance apart. Knowing the time of travel and the path length, the velocities through the material can be computed by using the following form \_a: 1

Pulse velocity, fps =  $\frac{\text{Path length, ft}}{\text{Effective time, sec}}$ 

The pulse velocity provides an index of the condition or quality of the concrete through which the readings are taken.

### Procedures

5. The procedure during the major part of the investigation was to take saity measurements straight through various walls, columns, and beams to determine the representative velocity of the concrete or find any cracked, shattered, or otherwise inferior concrete present at points where measurements were taken. Using appropriate combinations of the same points used for the straight-through measurements, diagonal readings were also taken through the concrete to locate inferior areas, particularly horizontal cracking, not detected by straight-through readings. If the signal must cross any horizontal or vertical cracking, a lower velocity is indicated. By comparing velocities obtained through damaged and repaired concrete with velocities representing undamaged concrete, an indication of the condition of the damaged or repaired concrete can be obtained.

<sup>\*</sup> Raised numbers refer to similarly numbered items in the Selected Bibliography following the text of this report.

PART III: TESTS AND RESULTS

### Soniscope Tests and Results

- δ. Results of the soniscope tests are given in tables 1-36. These
  tables include station numbers, station references, path lengths, velocities, remarks, and in some instances station location diagrams.

  R wall.
- 7. Sixty-five readings were taken on the R wall (figs. 2 and 3). The results are given in tables 1-3. Of the nine readings just east of wall 17, ground floor, only three had velocities greater than 9000 fps. This section had not been grouted and apparently contlined concrete of inferior quality. The R wall, main lobby, first floor, produced readings ranging between 12,145 and 13,740 fps. These readings indicate concrete of generally good quality. However, the problem of inaccessibility caused readings to be confined to a small portion of the wall. This section of the R wall had been grouted, and there is a possibility that some of the cracks were not completely filled. All readings taken on the R wall, first floor, between columns 12 and 13 near the garage entrance indicated concrete of generally good quality with the exception of four readings taken to locate an inferior area about 3 ft up from the floor near the entrance. R-wall beam
- 8. Eighty-three readings were taken on the R-wall beam at various locations; readings are given in tables 4-8. A section of the beam above the ground-floor lobby between walls 23 and 25, not grouted, produced three straight-through readings indicative of good concrete and eight diagonal readings indicating poor to questionable quality concrete (table 4). It was evident from the tests and from visual observation what extensive cracking was present in this beam. Twenty-four readings were taken on the R-wall beam, first floor, main lobby, between columns 23 and 25 (table 5). This section had been grouted. Only three readings were less than 10,000 fps, and 14 were more than 12,000 fps. The indications here are that although some cracks were apparently not filled, the general condition of the section ranged from questionable to good. The R-wall beam, first floor

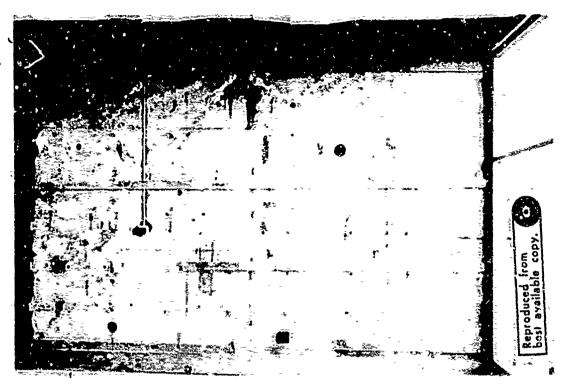


Fig. 2. R wall, main lobby, first floor



Fig. 3. Close-up of R wall, main lobby, showing scaler, grouting nonlle, and core holes

adjacent to wall 17, which had not been grouted, produced only 6 out of 15 straight-through readings above 12,000 fps (tables 6 and 7). Only three diagonal readings were greater than 10,000 fps; the remaining seven ranged between 4160 and 9215 fps. These readings indicate extensive cracking, inferior quality concrete, or both in this section of the beam. Two readings were taken through replaced concrete in this section of the beam, producing velocities of 15,625 and 16,395 fps. The third floor R-wall beam between walls 14 and 15 produced six straight-through readings great than 13,000 fps and two at 11,495 fps (table 8). Of the five diagonal readings taken, two were below 12,000 fps and three were below 9000 fps. Although this section had not been grouted, the straight-through readings indicated generally good quality concrete, and the diagonal readings indicated some cracking.

### Wall 17

9. One hundred and twelve readings were taken on various sections of wall 17 The results of these readings are given in tables 9-12. The ground-floor section of wall 17 immediately adjacent to the L wall and southward above the ceiling toward the kitchen seemed to be in generally good condition although some cracking was indicated. As reported in tables 9 and 10, all but two straight-through readings were above 12,000 fps. Only five of 15 diagonal readings were below 10,000 fps. This section had not been grouted. Seventy-seven readings taken on the grouted section of wall 17, first floor, above elevator door 5 (table 11) and opening into litters room A-183 (table 12) produced only three velocities below 10,000 fps. The remainder of the readings ranged between 16,520 and 14,916 fps, with 57 readings at 12,500 fps or above.

#### L wall

10. The results of soniscope tests made on the L wall are given in tables 13 and 14. All tests on the L wall were confined to a section at the ground-floor level. This section had not been grouted. Of 25 straight-through measurements made on the section by duct space 2, only two were below 13,500 rps. Twelve diagonals read in this area indicated some minor cracking. In an adjacent section of the L wall by stairway 1, three of 13 straight-through velocities were below 13,000 rps. The diagonal readings averaged 7540 rps, and indicated that this section

contained more severe cracking than the section by duct space 2. Wall 19 in duct space 2

11. Selective coring and results of ultrasonic testing revealed that full penetration by epoxy grouting had not been obtained in some cases. Therefore it was decided that one wall should be designated as a test panel; that cracks should be sealed on both faces of the wall; and that the wall should be pumped to refusal with epoxy grout, progressing from low to high elevation. A section of wall 19 in duct space 2 was chosen and the above procedure was followed (fig. 14). Pulse-velocity measurements were made on the test panel prior to grouting. 24 to 48 hr after grouting, and 72 to 36 hr after grouting (table 15). The 14 readings taken 24 to 48 hr after grouting showed velocity increases ranging between 0.6 and 48 percent, with an average increase of approximately 19 percent. The very small increases occurred at points where there was very little cracking and high velocities were obtained with the first reading. When compared with the 24- to 48-hr readings, the readings taken 32 to 96 hr after epoxy repair showed an average decrease in velocity of slightly less than 2 percent. This decrease after extended curing of the epoxy is not surprising in the light of results of the tests discussed in paragraph 23. The 24- to 48-hour readings taken at stations K, L, and M were at least 795 fps lower than the eleven other 24- to 48-hr readings. These readings showed an average increase of only 14 percent instead of the expected average increase of about 19-percent. Check reslings at these stations produced the same results. A core was taken through a visible crack near sta 4 on the line between sta A and sta K, L, and M. This core revealed a large unfilled crack between sta A and sta K, L, and M which accounts for the low readings obtained at these stations. The results obtained from velocity tests performed on this section of wall 19 indicate that significant increases in the ultrasonic pulse velocities can be effected in damaged concrete by epoxy grouting if the proper procedures are followed.

### Trird-floor spandrel beam

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i2. <u>Retween columns 13 and 14.</u> Twelve readings were taken on the interior section of the third-floor spandrel beam which had not been

Fig. 4. Wall 19 in duct space 2, test panel

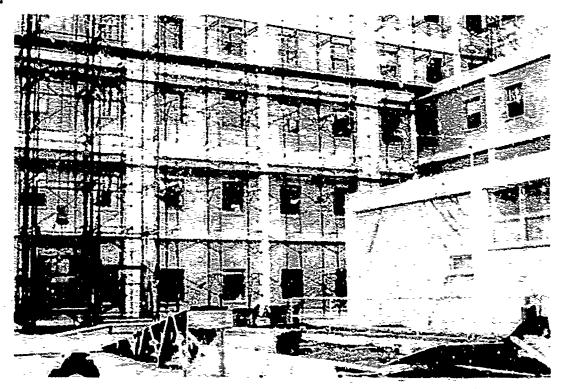


Fig. 5. Third-floor spandrel beam (first wide beam) and columns 8 (wide), 10, and 12 (wide)

grouted (table 16). The four straight-through readings indicated concrete of generally good quality. Velocities obtained by taking diagonal measurements indicated concrete quality from very poor to good. The low diagonal velocities indicated the presence of severe cracking.

- 13. Beam and column readings, north side. Results of readings designed to determine the general condition of beams, columns, and column-beam connections along the entire third-floor spandrel beam between columns 2 and 13. north side, are given in table 17. This entire section had been grouted. All straight-through readings were above 14,000 fps with the exception of one reading of 13,765 fps. Most of the diagonal readings on this section ranged between 10,000 and 15,000 fps. The high and low readings within this range seemed to be well distributed along the beam with no particular grouping pattern. These readings (10,000 to 15,000 fps) indicated conditions ranging from partially filled cracks to undamaged concrete. Seven diagonal readings were below 10,000 fps. Three of these readings, which were significantly low, were taken through the east column-beam connection at column 8. Unrepaired cracking was suspected at this connection. Two of the low readings were obtained in a saction of the spandrel beam between column 2 and about midway of the beam west of column 2.
- 14. Readings were taken through the outer 6 in. of column-beam connections at columns 8, 10, and 12 (fig. 5) to determine the extent of penetration in the outer face. The locations of these stations and the result of tests are recorded in tables 18, 19, and 20.
- 15. Beam and column readings, south side. Readings similar to those taken on the third-floor spandrel beams and columns on the north side were also taken on the south side of the building. However, due to inaccessibility, readings were confined to the vicinity of the column-beam connections. Columns and beams had seen grouted up to column 8; no grouting had been accomplished at columns 8, 10, 12, and 14. The grouting seemed to make very little difference in the general condition of the column-beam connection readings. This fact is evidenced in the following tabulation.

Column	Average Reading, fps	Remarks
4* 6* S 10 12	10,835 13,275 13,020 12,380 12,110 12,160	Four of 12 below 10,000 fps One of 12 below 12,000 fps Three of 12 below 12,000 fps Five of 20 below 10,000 fps Three of 24 below 10,000 fps One of 12 below 10,000 fps

<sup>\*</sup> Grouted.

At three of these columns, readings indicated that the column-beam connections on one side of the column contained more cracking than the other side. This condition was indicated on the west side of columns 14 and 8, and on the east side of column 10. Station locations and results for this section are given in tables 21-26.

16. Column-beam readings through the outer 6 in. were taken at column 10 to check for penetration of epoxy. All readings produced velocities above 13,000 fps. Station references and results of readings are given in table 27.

### Fifth-floor spandrel beam and column connections, north side

17. Velocity measurements were made through the cuter 6 in. of column-beam connections on the fifth floor, north side of building, at columns 12 and 10. Results (tables 28 and 29) indicated these connections to be very good at column 10. Three of four measurements at column 12 indicated a good condition.

### East stairway wall 2 between wall T and fan room wall

18. Readings were taken in this section on the first, second, third, and fourth floors. Results of the tests in this area are given in tables 30-33. Extensive cracking was indicated on the first floor along with a trend toward less extensive cracking in the higher floors. Most severe cracking occurred around door openings.

# Wall 13, mechanical equipment room B-254A

19. Results of the 29 readings taken in this area are given in table 34. The eight diagonal readings taken in the area between the large ducts ranged between 11,710 and 15,120 fps, indicating that some cracking

was present. The absence of any very low readings seems to indicate that the cracking was of a minor nature. Severe cracking was indicated near the south side of the door and in an area above the south side of the door.

## Column beam connections around outpatient clinic

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20. Eight readings were taken at five exterior columns around the outpatient clinic (fig. 6) in an attempt to determine the effectiveness of the epoxy repair. Inaccessibility of interior faces of these columns and beams limited the type and number of readings that could be taken. The location and results of these readings are given in table 35.

### Interior column 6 at S wall

21. Two measurements were taken on this interior column for the purpose of comparing the velocities with those of exterior columns. Velocities of 13,700 and 13,940 fps were obtained. The velocities of exterior columns on the main shear walls generally ranged between 14,500 and 16,000 fps.

### Second-floor slab

22. Three readings were taken in a patched area of the floor slab, second floor, near elevator 5. The velocity in the new concrete was 14,540 fps; the velocity in the old concrete was 12,500 fps; the velocity from old to new was 7550 fps. This low velocity from old to new concrete indicates a poor bond between the old and new concrete.

### Epoxy-repaired cores

pair on pulse valocity was performed on three 4-in.-diameter cores taken from the L wall, ground floor. These cores were broken transversly, and then repaired with layers of apoxy of different thicknesses. One core contained only a film of epoxy to form the bond. The other two were repaired with 1/15- and 1/8-in. layers, respectively. After repair, the specimens were cured for 24 hr at a temperature between 70 and 80 F, and then dried in an oven for 24 hr at a temperature of 150 F. The temperature was then reduced to 120 F for a period of 43 hr. Velocity measurements were made on the cores before breaking and after each phase in the curing process. To obviate any effect that drying of the specimens

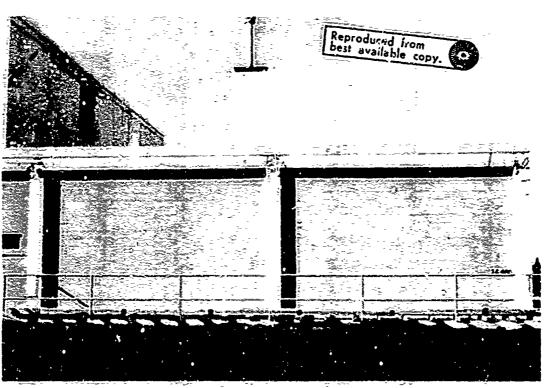


Fig. 6. Fast wall of outpatient clinic area showing column C, column B, and corner column A

might have on pulse velocity, the cores were saturated with water for an initial reading prior to breaking and the final reading after repair. The results of these tests (table 36) show only a 5.6 percent average decrease in pulse velocity after breakage and epoxy repair. A decrease of 3 to 4 percent in pulse velocity occurred between the readings taken after 24 hr of curing at room temperature and the readings taken after accelerated curing in the oven. This decrease seems to be in line with a slight decrease noted in velocities obtained in tests performed on wall 19 when the epoxy was at about these same ages (see paragraph 11).

### Other Tests and Results

### Compressive strength

24. A 4-in.-diameter core (core 1) taken from the R wall, main lobby, first floor, and a similar core (core 2) taken from the L wall by stairway L, ground floor, were taken to the Concrete Branch testing laboratory, Alaska District, for compressive strength tests. The specimens were capped and tested in accordance with CRD-C 14-63. Polse velocity measurements were made on the cores prior to capping and breaking. Results of these tests are given in the following tabulation.

	Compressive Strength	Pulse Velocity
Core	psi	fps
1	4876	13,870
2	5444	14,880

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The liwited amount of data obtained from these tests precludes the establishment of any reliable correlation between pulse velocity and compressive strength. However, these data conform to the generally expected pattern of a higher pulse velocity with a higher compressive strength.

### Tensile splitting

25. Tests were performed on eight 2-in.-diameter cores to compare the tensile-splitting strength of the epoxy bond in cracked concrete with that of the undamaged concrete. Procedures outlined in test method CRD-C 77-61 were followed with the exception that 2-in. cores were tested.

The results of these tests are given in the following tabulation.

Specimen	Tensile-Splitting Strength, psi
1	588.4
10	441.0
2C	682.4
3C	494,4
1E	822.0
2E	861.4
3E	665.5
4E	378.5

Specimen 1 was part of an undamaged core taken from the test panel, wall 19. Specimens 1C, 2C, and 3C were cut from a single core of repaired concrete taken from the same wall. Specimens 1E-4E were cut from a single core of repaired concrete also taken from the test panel, wall 19. Although the epoxy-repaired cracks did not run entirely parallel to the specimen axes, the specimens were loaded as nearly as possible along the repaired openings. Four of the seven repaired specimens failed at strengths considerably greater than that of the undamaged specimen. The other three showed strengths from 15 to 35 percent less than that of the undamaged specimen. However, the most significant fact derived from these tests was that the break in each repaired specimen occurred along the axis of the specimen and not along the line of epoxy repair. This indicates that the epoxy bond was at least as strong as the adjacent undamaged concrete in the specimen.

### PART IV: CONCLUSIONS

- 26. Representative velocities of good concrete in the L wall, spandrel beams, and columns ranged between 14,000 and 15,000 fps. Other areas exhibited velocities between 13,000 and 14,000 fps in good concrete. All sections tested exhibited some low velocities in straight-through as well as diagonal readings. Low velocities were far more prevalent in ungrouted sections.
- 27. Significant increases in ultrasonic pulse velocities can be effected in damaged concrete by epoxy grouting if proper procedures are followed. However, velocities obtained after repair will probably be about 6 percent lower than velocities in the original undamaged concrete. Sealing both faces of an area of concrete and pumping so refusal while progressing from low to high elevations seemed to be the best method for attaining effective repair by epoxy grouting in the areas tested during this investigation.
- 28. Results of compressive strength-pulse velocity tests, though limited, indicate that the compressive strength of the concrete in the hospital building probably ranges between 4000 and 5000 psi, and that concrete of this quality will probably produce pulse velocities ranging between 13,000 and 15000 fps. Results of tensile-splitting tests indicate that cracked concrete when properly repaired with epoxy grout is as strong as the adjacent original concrete.

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- 29. It is indicated from the test results that there are areas within the structure where repair by epoxy grout is not complete. There is some question of whether further penetration c i be obtained in areas that have already been pumped unless pumping is done from the opposite face.
- 30. It is further concluded that ultrasonic testing is a rapid, economical, and satisfactory method of detecting gracked or otherwise inferior quality concrete, and that indications of the state of repair of the concrete can be derived from test results.

### SELECTED DIBLIOGRAPHY

- U. S. Army Figineer Waterways Experiment Station, CE, <u>Handbook for Concrete and Cement</u>, with quarterly supplements. Vicksburg, <u>Miss.</u>, August 1949.
- 2. , Comparative Tests of Schlscopes for Ultraschic Testing of Concrete, by R. V. Tye, E. J. Callan, and E. C. Roshore. Miscellaneous Paper No. 6-48, Vicksburg, Miss., September 1953. (With 19 references.)
- 3. Field Soniscope Tests of Concrete: 1953 Tests, by
  E. C. Roshore. fechnical Memorandum No. 6-385, Report 1, Vicksburg, Miss.,
  April 1954.
- 4. , Ultrasonic Investigation of Cracking in Kansas City
  Floodsall (Armourdale Unit), by E. C. Roshore. Miscellaneous Paper No.
  6-72, Vicksburg, Miss., May 1954. (With 6 references.)
- 5. Field Investigation of Concrete, Texarkana Reservoir
  Project. Miscellaneous Paper No. 6-120, Vicksburg, Miss., March 1955.

  (With 4 references.)
- by Bryant Mather. Miscellaneous Paper No. 6-235, Vicksburg, Miss., August 1957. (With 11 references.)
- 7. Field Soniscope Tests of Concrete; 1953-1957 Tests, by E. C. Roshore. Technical Memorandum No. 6-383, Report 2, Vicksburg. Miss., March 1958.

THE TANKE HE SENDE AND SEN

- 8. Investigation of Compressive Strength of Molded Cyli ders and Drilled Cores of Concrete. Technical Report No. 6-522, Vicksburg, Miss., August 1959. See also "Investigation of compressive strength of molded cylinders and drilled cores of concrete," by Bryant Mather and W. O. Tynes. Proceedings of the American Concrete Institute, vol 57 (January 1961), pp 767-778.
- 9. , Velocity Tests of Concrete, Riverside Wall, Asnley Street
  Plant, Union Electric Company, St. Louis, Missouri, by J. H. Sanderson.
  Miscellaneous Paper No. 6-456, Vicksburg, Miss., October 1961.
- New York, by E. C. Roshore. Miscellaneous Paper No. 6-527, Vicksburg, Miss., September 1962.
- Auxiliary Sites, Squadron III, by H. T. Thornton, Jr. Miscellaneous Paper No. 6-58), Vicksburg, Miss., July 1963.

Results of Soniscove Tests
R Wall East of Wall 17 Between Ground and 1st Floor
(Not Grouted)

	Station R	eferences				والمراوية
	Distance	Distance	T . 1		ities	
A	bown from	East from	Path		'ps	
Station Numbers	lst Ploor in.	Wall li	Length ft	Straight Through	Diagonals	Remarks
	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	Company on the party of the party of		11/7 00/21	12 × 142 (11.270)	
83	40	7				Only one side accessible
85	20	50				Only one side accessible
87	60	50				Only one side accessible
88	20	40				Only one side accessible
89	40	40	1.00	12,660		
90	60	j40	1.00	9,345		
91	20	60				Only one side uccessible
92	<del>,</del> †O	60	1.00	12,985		-
93	40	100				Only one side accessible
<u>5</u> 4	бо	160				Only one side accessible
83 to 90			3.32		3905	
87 to 89			2.54		7470	
85 to 89			2.54		5423	
88 to 92			2.54		5315	
91 to 94			4.85		5160	
92 to 93			3.48		7280	

Table 2

Results of Soniscope Tests

R Wall, Main Lobby, 1st Floor

(Grouted)

	Station Re	The second secon		1	
	Distance	Distance	Path		cities fps
Station Numbers	from East Face in.	from Floor in.	Length ft	Straight Through	Diagonals
101	28	112	1.00	13,700	
102	34	78	1.00	13,330	
103	34	67	1.00	13,150	
10/1	34	35	1.00	12,660	
105	<b>3</b> 5	17	1.00	13,330	
106	20	112	1.00	12,500	
107	24	61	1.00	13,150	
108	23	35	1.00	13,330	
109	8	109	1.00	13,150	
130	8	68	1.00	13,330	
101 to 109			1.98		12,145
102 to 109			3.64		12,860
102 to 110			2,58		12,525
104 to 110			3.64		13,095
1.05 to 108			2.02		13,740

Table & Result: of Confecçõe Tests

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R Wall, 1st Ficor Between Column: 12 and 13 East of outside Entrance Next to Garage (Growtel)

Station Number.	Sation 51. tance At ove Hole' Center, in.	Heforence Dictagos Center, West	F: 5%	Fith Leigth ft	Velo it Etraight Through	des, ip. Diagonal.	Rezurz.
.20	71	£		1.00	13,570		
137	11			1.00	15,730		
y mit	0			1.00	12,400		
12)	21	Š		2.00	13,590		
130	14	ي .		1.30	13,515		
¥3°.	1		23	1.00	14,085		
132	3		10	1.00	المجعرورا		
<u> </u>	<u></u> :2		20	1.00	12,~30		
<u>₹</u> 2°,	•		20	1.∙∿	1,190		
135	<u>- 1</u>		6	1.00	13,590		
136	15		34	1.00	13,53		
-37	э		312	1.90	13,590		
138	31		43	1.00	14.550		
139	3.5		43	1.33	14,230		
149	Ģ		43	1.00	1.,2%		
127 to 148				1	-	24,735	
128 to 179				1.51		14.547	
131 to 132				1.42		13.150	
133 to 134				1.42		15,150	
129 to 135				2, 77		13.3.5	
12 to 130				77		1.15	
139 to 240				1.43		13.7°5	
Re	ading: Taken	to lecate	Stree	etei ini	brior Car	nete	
		in One Se	ction	of Wail			
Station 134 to points along				2.14		12,2	
an are 3.1% ft from sta-		-		£. 14		13,69	Indicate, this
tion 13- serves suspected area				2.14		13.9% 13.3%	arsek fillet
Station 134 to points along				4,00		12,620	Indiester empara-
an are 4.0 ft from sta- tion 134 across suspected				4.00 4.00		,530	tively interior
una				4.00	**	11,e30 13,20.	area avent i ft up frar ficer
				4.30		1.592	nese entrace
Station 120 to points along the are 16 in. below sta-				1.67		13,500	Tedinosi = (
tion 123 in good wrea just	:			1.67 1.67		13,465 13,465	Indication: great
above suspected area							
Station 128 to points along an are 3.04 ft below sta-				3.20 3.20		13.85; 13,855	Indications 300:
tion 128 juse bolow sus- proted area				3.30		12,k)	Hereasson, Gra-
Station 12° to points along				2.3≃		12,865	
on are 2.17 it below sta- tion 128 in suspected area				2.3± 2.3±		12,863 13, <i>9</i> 00	
eron the in suspected at 60				2. <u>yz</u> 2. <u>3</u> ż		15,000	

When also a more consistent with the solution of the solution

<sup>•</sup> Stations 126 through 140 were referenced from the center of a 4-in. core hole brilled through the wall. The center of the core hole was 65.5 in. up from lot floor and 13.5 in. east of wall 13.

Table 4

Results of Soniscope Tests
R-Wall Beam Above Lobby, Ground Floor
Between Walls 23 and 25

(Not Grouted)

	Station F Distance up from Bottom	Distance West from	Path		ities ps	
Station Numbers	of Beam	Wall 23 in.	Length ft	Straight Through	Diagonals	Remarks
106	37	21	1.08	14,400		
106A	37	25 east of 106				Only one side ac-cessible
107	17	21				Only one side ac-cessible
108	24	71	1.08	13,500		
109	8	95				Only one side ac-cessible
110	24	108				Only one side accessible
111	40	133	1.08	13,170		
112	40	170				Only one side ac-
113	10	165				Only one side ac-cessible
106 to 106A			2.35		11,520	
106 to 108			4,46		9.510	
107 to 108			4.31		11,370	
108 to 109			2.58		7,565	
109 to 111			4.14		7,765	
111 to 112			2.95		11,345	
111 to 113			3.90		8,495	
110 to 111			2.72		7,575	

Results of Soniscope Tests
R-Wall Beam Over 1st Floor Opening (Main Lobby)
Retween Columns 20 and 25

(Grouted)

是这个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就

		References	,		
	Distance	Distance	Path		cities
Station	from Top of Beam	West from Wall 23	racn Length	Straight	î pe
Numbers	in.	in.	<u>rt</u>	Through	Diagonals
111	8	7	1.00	12,050	
112	16	7	1.00	12,990	
113	24	7	1.00	13,160	
114	11	23	1.00	13,160	
115	19	23	1.00	13,890	
116	26	23	1.00	12,195	
13.7	12	89	1.00	11,630	
311	20	89	1.00	10,525	
119	9	96	1.00	13,485	
120	15	96	1.00	13,955	
121	23	96	1.00	13,185	
122	17	132	1.00	14,490	
123	8	174	1.00	12,345	
124	17	174	1.00	13,160	
125	5 <i>i</i> 4	174	100	12,820	
119 to 122			3.00		10,675
120 to 122			3.17		11,830
121 to 1∠2			3.57		10,230
111 to 116			2.1.7		7,210
112 to 115			1.60		11,345
117 to 118			1.25		9,540
122 to 125			3.64		10,225
122 to 124			3.64		15,825
122 to 123			3.72		8,435

Results of Soniscope Tests
R-Wall Beam, 1st Floor Adjacent to Wall 17
Between Columns 16 and 1.7
(Not Grouted)

<del></del>	Station	References	<del></del>		
	Distance	Distance	way i t		cities
Station	from Wall 17	from Floor Above	Path Length	Straight	fps
Numbers	in.	in.	ft	Through	Diagonals
172	3	SJ -	1.00	11,630	
173	3	39	1.00	10,990	
174	3	51	1.00	10,990	
175	15	<u> 21</u>	1.00	10,640	
176	15	39	1.00	9,900	
177	15	. 51	1.00	13,700	
178	24	51	1.00	10,755	
179	24	39	1.00	13,160	
180	54	51	1.00	12,500	
181	44	21	1.00	12,195	
182	44	39	1.00	12,500	
183	1414	51	1.00	12,660	
172 to 176			2.02		6,435
175 to 179			2.05		7,195
174 to 177			1.42		11,360
180 to 182			8.23		9,215
179 to 183			8.23		8,545
178 to 182			2.53		6,875
179 to 181			2.47		11,175
181 to 184			1.80		6,520
181 to 185			2.17		6,975
	Wo Reading	s Taken Throu		d Concrete	
to.		in R-Wall			16 200
F			1		16,395
G			ı		15,625

Results of Soniscope Tests
R-Wall Ream, 1st Floor Between Columns 16 and 17
(Not Grouted)

	Station Distance East of Opening	References Distance Bown from Floor	Path		cities fps
Station Numbers	in Beam in.	Above in.	Lergth ft	Straight Through	Diegonals
1001	15	40	1.00	11,365	
1002	24	57	1.00	10,310	
1003	31	55	3.00	6,370	
1001 to 1002			1.95		10,315
1001 to 1003	-		2.08		6,730
Read	lings from Sta 46 in	ation 1002 to East of Sta		long an Arc	
1002 to 1004			3.98		8,075
1002 to 1005			3.98		4,160
1002 to 1006			3.98		5,190
1002 to 1007			3,98		6,495
1002 to 1008			3.98		6,150
1002 to 1009			3.98		6,105
1002 to 1010			3.98		8,010

Table 8

Results of Soniscope Tests

R.Wall Beam Between Walls 14 and 15

Between 2d and 3d Floors

(Not Grouted)

	Station F Distance from East Side	eferences Distance	·		
	of Large	Down from	<b>-</b>	Veloc	
Station	Puct Opening	3d Floor	Path Length	Straight	ps
Humbers	in.	in.	ft	Through	Diagonals
501	40	20	1.00	13,335	
202	40	40	1.00	13,335	
203	80	20	1.00	13,160	
204	80	40	1.00	13,700	
205	120	50	1.00	13,160	
205	120	40	1.00	13,160	
207	160	20	1.00	11,495	
208	160	40	1.00	11,495	
201 to 204			3.88		11,280
202 to 203			3.88		7,670
203 to 206			3.88		10,265
204 to 205			3.88		7,565
206 to 207			3.58		8,170

Table 9

Results of Soniscope Tests

Wall 17 Adjacent to L Wall Between Ground and 1st Floor

(Not Grouted)

-		<u>leferences</u>				
	Distance	Distance				
	Down	from	<b>5</b> (3)	Velocities		
Chatian	from 1st	North	Path		ag	
Station Numbers	Floor in.	Face, R Wall,in.	Length ft	Straight Through	Diagonals	Remarks
Mandelo	244-	10111		11204611	Diagonard	1(0)1107
40х	50	293		13,890		Only one side accessible
43.	20	293				Only one side accessible
42	37	293	1.00	13,890		
43	57	293				Only one side accessible
45	20	265				Only one mide accessible
45	37	265	1.00	13,515		
47	57	265	1.00	13,890		
49	20	555				Onl, one side accessible
51	57	222				Only one side accessible
57	57	183				Only one side accessible
40X to 41			1.95		9,375	
41 to 47			2.38		7,040	
12 to 47			3.00		12,875	
42 to 45			2.64		12,455	
43 to 46			3.08		14,810	
45 to 51			4.70		13,705	
49 to 57			4.62		8,265	

Table 10

Results of Soniscope Tests
Wall 17 in Duct Space 2

(Not Grouted)

	Station Re	1erences			
	-	Distance			
	Distance	from North			_
	from Ground	Face of	Path	Velociti	es, îps
Chalden Numbous	Floor	I, Wall	Length	Straight	Nonevola
Station Numbers	in.	in.	<u>ft</u>	Through	Diagonals
95	102	50	1.00	13,335	
96	79	50	1.00	11,495	
97	62	50	1.00	11,495	
98	42	50	1.00	12,050	
99	102	දිර	1.00	14,085	
100	79	<b>63</b>	1.00	13,700	
101	62	80	1.00	i3,515	-
102	42	80	1.00	13,335	
103	102	110	1.00	13,890	
104	102	140	1.00	14,705	
105	102	180	1.00	14,705	
95 to 99			2.75		9,965
96 to 99			3-33		11,935
96 to 101			2.84		11,935
97 to 102			3.17		12,780
98 to 101			3.17		9,875
100 to 103			3.23		14,815
103 to 104			2.75		12,500
104 to 105			3.48		12,385
_					

Table 11
Results of Soniscope Tests

### Wall 17 Adjacent to R Wall Between 1st and 2d Floors

Above Elevator Door 5

(Grouted)

		eferences	•		
Station	Distance from	Distance from Floor	Fath Length	Velocit Straight	ies, fps
Numbers	R Wall, in.	Above, in.	ft	Through	Diagorias
141	10	10	1.00	12,500	
142	10	18	1.00	12,500	
143	10	42	1.00	12,820	
ፓ <del>ሳ</del> ት	10	54	1.00	12,500	
145	32	10	1.00	12,500	
146	32	18	1.00	12,820	
147	32	42	1.00	12,500	
148	32	54	1.00	12,500	
149	1414	10	1.00	12,985	
150	fift	18	1.00	12,985	
151	7474	42	1.00	11,495	
152	प्री	54	1.00	12,050	•
153	59	10	1.00	13,890	
154	59	18	1.00	13,890	
155	59	42	1.00	10,870	
156	59	2 <del>ji</del>	1.00	12,500	
142 to 146			2.08		12,235
143 to 147		~	2.08		12,235
145 to 149			1.42		11,640
147 to 151			1.42		11,640
146 to 150			1.42		9,660
149 to 153			1.60		12,030
150 to 154			1.60		12,500
151 to 155			1.60		9,940
152 to 156	-		1.60		10,740
150 to 155			2.50		12,565
152 to 155		-	1.99		13,355
142 to 147			2.88		10,705
147 to 149					12,90
		(Continued	)		

Table 11 (Concluded) -

	Station R	eferences	·	<del></del>	
01 - L2	Distance	Distance	Path		ies, fps
Station Numbers	from R Wall, in.	from Floor Above, in.	Length ft	Straight Through	Diagonals
186	71	10	1.00	12,820	
187	71	18	100	14,085	
188	71	42	1.00	12,500	
189	71	514	1.00	13,515	
190	93	10	1.00	13,160	
191	93	18	1.00	12,050	
192	93	42	1.00	13,160	
193	93	2 <u>j</u>	1.00	12,820	
194	117	10	1.00	12,820	
195	11.7	18	1.00	12,820	
196	117	42	1.00	12,820	
197	11.7	54	1.00	13,335	
190 to 194			2.23		13,435
191 to 195		=	2.23		10,520
187 to 192			2.47	•	13,645
192 to 196			2.23		14,295
196 to 197			1.42		11,450
190 to 191			1.20		13,150
187 to 188			2-23		11,265
188 to 189			1.42		12,455
155 to 156			1.42		7,170
155 to 187			2.47		11,705
	Readings Thr	ough Replaced C	oncrete in	South	
	<u> </u>	of Door, Litters		•	
A	12	10	1.00	14,570	
В	24	12	1.00	14,570	
C	36	10	1.00	14,510	
D 	48	10	1.00	14,510	
E	60	10	1.00	14,510	
A to E			4.15		15,090

Table 12
Results of Soniscope Tests

Wall 17 Between 1st and 2d Floors Above Opening into Litters Room A-183 (Grouted)

	Station R Distance from North	eferences Distance from Floor	Path	Velocities fps	
Station Numbers	Wall in.	Above in.	Length	Straight Through	Diagonals
157	10	12	1.00	13,700	
158	10	31	1.00	14,085	
159	10	55	1.00	13,700	
160	51	12	1.00	14,285	
161	51	31	1.00	13,890	
162	51	55	1.00	13,160	
163	36	12	1.00	14,285	
164	36	31	1.00	13,700	
<u>1</u> 65	36	55	1.00	13,700	
166	53	12	1.00	13,700	
16?	53	31	1.00	13,890	
168	53	55	1.00	14,085	
169	77	12	1.00	15,150	
170	77	31	1.00	14,285	
171	77	55	1.00	13,700	
167 to 169			2.83		13,605
166 to 170			2.73		12,295
168 to 170			3.00		13,335
167 to 171			3.00		14,285
164 to 168			2.62		14,475
164 to 165			2.23		13,435
163 to 164			1.98		13,240
157 to 164			2.97		13,260
158 to 162			2.39		13,060
159 to 161			2.39		13,060
160 to 163			1.67		14,910

Table 13

Results of Soniscope Tests

L Wall by Duct Space 2

Ground Floor Level
(Not Grouted)

	Station I	erences			
	Distance from East Face	Distance up from	Path	Velocities fps	
Chatian Membana	Wall 17	Floor	Length	Straight	11/ 1 -
Station Numbers	<u>in.</u>	in.	<u>ft</u>	Through	Diagonals
1	56-1/4	114	1.00	14,495	
2	56-1/4	97	1.00	14,495	
3	56-1/4	85	1.00	14,495	
4	56-1/4	65	1.00	14,085	
5	56-1/4	49	1.00	13,890	
6	711/4	114	1.00	14,495	
7	71-1/4	97	1.00	14,495	
8	71-1/4	Ö5	1.00	14,495	
9	711/4	65	1.00	13,890	
10	71-1/4	49	1.00	13,700	
11	86-1/4	114	1.00	14,085	
12	86-1/4	97	1.00	14,710	
13	86-1/4	85	1.00	14,085	
14	86-1/4	65	1.00	13,890	
15	86-1/4	49	1.50	14,285	
16	101-1/4	114	1.00	14,085	
17	101-1/4	91	1,00	14,925	
3,5	101-1/4	85	1.00	14,495	
19	101-2/4	<del>5</del> 5	1.00	12,820	
20	101-1/4	49	1.00	12,195	
Sī	110-1/4	114	1.00	13,515	
55	110-1/4	97	1.00	14,085	
83	110-1/4	85	1.00	14,285	
Ş4	110-1/h	65	1.00	14,085	
25	110-1/4	49	1.09	13,700	
	•	*		- • ·	

internation who were the anti-standing of the control of the contr

(Costinued)

Table 13 (Concluded)

		eferences			- Carlotte of Brillian School Section
	Distance from East Face	Distance up from	Path		cities fps
Station Numbers	Wall 17	Floor in.	Length <u>ft</u>	Straight Through	Diagonals
1 to 8			2.34		11,270
3 to ó			2.84		12,085
3 to 10			3.48		9,750
3 tc 8			3.48		11,715
6 to 13			2.93		11,015
8 to 11			2.84		9,190
8 to 15			3.48		12,565
10 to 13			3.48		12,125
10 to 23			3-33	•	8,495
13 to 21			3.33		12,020
13 to 25			3,80		10,735
15 to 23			3.80		11,985

Table 14

Results of Soniscope Tests

L Wall, Ground Floor by Stairway 1

(Not Grouted)

	Station Re	ferences			
Scation Numbers	Distance from East Face Wall 17 in.	Distance up from Floor in.	Path Length ft	Velocit Straight Through	ies, fps Diagonals
56 Manager 2	1.80	114	1.08	10,000	
27	180	85	1.08	10,485	
28	180	49	1.08	10,095	
29	240	114	1.08	13,670	
30	240	85	1.08	14,025	
31	240	49	1.08	13,670	
. 32	3 <del>7</del> 0	13	1.08	14,025	
34	300	114	1.08	13,335	
35	300	85	1.08	13,170	
36	300	13	1.08	14,595	
38	348	85	1.08	13,670	
39	348	49	1.08	13,500	
40	348	13	1.08	13,170	
25 to 26			9.30		6,815
22 to 28			7.89		7,108
26 to 30			4.77		8,565
28 to 30			5.10		7,390
27 to 29			4.77		8,265
27 to 31			5.10		6,735
29 to 34			5.59		6,600
31 to 34	-		5.92		6,400
34 to 39			5.18	_	10,635
35 to 38			5.10		8,715
30 to 35			5.92		5,690
			-		

Table 15

Results of Soniscope Tests

Wall 19 in Duct Space 2

(Test Parel)

Station Desig- nation	Path Length ft	9/2/64 Before Repair	24 to 48 hr After Repair	72 to 96 hr After Repair
<b>A*</b>	1.00	12,660	14,285	13,515
A to B	2.08	11,125	13,080	13,165
A to C	2.17	10,795	13,310	12,615
A to D	2.84	6,650	12,680	12,085
A to E	2.31	7,175	12,905	12,625
A to F	3.00	9,375	13,160	12,820
A to G	3.48	9,720	12,795	12,935
A to H	3.64	14,108	14,445	14,680
A to I	3.48	12,745	14,380	14,320
A to J	3.96	10,180	13,380	13,070
A to K	2.53	8,665	11,345	11,450
A to L	2.08	11,685	11,885	12,310
A to M	2.30	9,745	11,560	10,550
A to N	2.01	13,225	13,310	13,225

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<sup>\*</sup> All readings taken diagonally through wall except at point A where straight-through readings were taken. Point A located 90 in. from ground floor and 42 in. south of L wall.

Table 16

Results of Soniscope Tests

3d Floor Spandrel Beam

Between Columns 13 and 14

(Not Grouted)

(North Side)

	Station R Distance	eferences Distance	•			
	from	up from	Dath		ities	
Station	West End of Heam	Bottom of Beam	Path Length		ps	
Numbers	in.	in.	ft	Through	Diagonals	Remarks
209	13	17	1.17	14,625		
210	13	3	1.17	13,295		
21.1	43	17	1.17	13,765		
212	43	3	1.17	13,765		
213	83	15-1/2				Only one side accessible
214 (13-14)*	83	3				Only one side accessible
203** to 209			4.12		3,680	
208 to 210			3.64		6,895	
209 to 211			2.75		12,010	
209 to 212			2.99		10,275	
212 to 213			3.69		7,305	
211 to 213			3-53		12,050	
214 to BT	53 east of point 214	25-1/2	4.58		8,910	
214 to BU	49 east of point 214	5-1/2	4.25		10,045	

<sup>\*</sup> Numbers in parentheses indicate columns between which the station was located.

<sup>\*\*</sup> On R-wall beam, see tah. 8.

Tuble 17
Lesuits of Soniscope Tests

3d Floor Spandre: Beam; Readings Through Powns, Columns, and

Column-Benz Councctions

(Grocted)

(North Side)

	Station Reference Distance from West End of Beam	Distance up from Bottom of b. sa	Path Lengtn	Velocities, fps Straight
Station Numbers	in.	in.	raci. Rengen	Through Diagonats
215 (12-13)•	165	15-1/2	1.17	14,095
216 (on col. 14)	227	δ	1.17	14,0.5
215 to 216			5.36	10,055
215 to BT	34 west of point 215	z5-1/z	3.07	11,610
215 to BU	3h west of point 215	5-1/2	3.07	.4,550
215 to BK	36 west of point 216	25-1/2	3.84	13,52:
216 to BS	36 west of point 216	5-1/2	3.2.	14,440
216 to BP	42 cust of point 216	25-1/c	3.59	13,665
216 to BQ	42 east of point 216	5-1/2	3.69	13,570
217 (10-12)	299 east of col 13	15-1/2	1.17	14,695
218 (10-12)	362 east of col 13	15-1/2	1.17	14,445
219 (10-12)	428 east of col 13	15-1/2	1.17	14,625
229 (on col 10)	500 east of col 13	15-1/2	1.17	13,705
221 (8-10)	566 east of col 13	15-1/2	1.17	14,465
217 to BP	35 west of point 217	25-1/2	3.14	14,215
217 to BQ	32 west of point 217	5-1/2	2.91	14,050
217 to 218	-		5.30	12,010
218 to 219			5.62	14,190
219 to BV	37 cust of point 219	25-1/2	3.3C	14,693
219 to 30	37 cast of point 2'9	2-1/د	3-30	12,515
220 to EN	38 west of point 0	25-1/2	3∙3₺	11,5775
220 to R0	38 west of point 220	5-1/4	3.30	13,575
220 to BL	13 east of point 20	<i>2</i> 5-1/2	1.57	11,545
220 to BM	13 east of point A20	5-1/4	1.,1	14,765
220 to 22).			>.<3	12,485
221 to BJ	43 gast of point 3_1	_>-1/≤	કર્ના	12,525
221 to EK	43 east of point cal	5-1/-	3-11	14, 45
?22 18-10 <b>)</b>	647 east of col 13	15-1/=	1.17	14,625
223 (8-10)	/16 east of col 13	15-1/5	1.1.	14,005
224 (middle col 8)	750 east of col 13	13-1/	1.17	15,000
222 to RJ	40 west of point and	ಪ್ರ-ಚ/ಪ	3-52	13,455
222 to IX	40 west of point dia	5-3/4	3-5-	9,030
222 to 223			5.85	14,005
223 to 6A		ين-1/a	2.03	11,405
223 to 68 On	On a 20-in. are east	15-1/4	<b>03</b>	12,630
22) to 80   601	of point 223	5-1/4	4.05	13,935
225 (3-8)	32 east of center, col 8	15-1/4	1.17	15,600
226 (6-8)	112 cust of center, col d	15-1/~	1.17	15,395
227 (6-8)	212 east of center, col c	15-1,2	1.17	15,000
2-8 (center, col 6)	257 cast of center, col ô	11-1/2	1.17	lo, vc°
		entirued)		

Mumbers in parentheses indicate columns tetween which the stations were located.

Table 17 (Concluded)

	Station Reference Distance from	Dialice up		
Station Numbers	West End of Pess in.	from Potton or Boam in.	Path Length ft	Velocities, fis Strught Through Disgovale
225 to 224A ) on		.5·1/2	2.91	6,395
25 to 224E , 201	On a 32-in. are west of	15-1/2	2.91	5,995
225 to 221/C \ 8	point 25	-2-1/2	2.91	7,645
?≥ó to B4	30 west of point 220	ع/-1/غ	£-75	14,4 5
ãó to BI	30 west of point 220	5-1/2	12	14,4±
627 to 828			Ĵ (c	13, 05
227 to ce3A	45 eact of point call	≤5-1/≤	3.92	14,200
22b to BF	55 east of point 220	z/-1/c	3.34	14,411
12ê to ki	43 evet of point 214	5-1/2	5-77	بخيرك
29 (4.6)	102 east of center, col 6	1/2	5.17	19,250
230 (4-6)	170 east of center, col c	15,-1/2	ž-4+	17,600
31 (on col 4)	240 east of center, col 6	13-1/2	1.1	a",13%
으 (2-4)	303 east of center, col t	17-1/2	1.17	14,44>
33 (2-4)	464 east of center, col é	15-1/2		
34 (2-4)	496 east of center, col c	10-1/2		
29 to BF	57 west of point 209	25-1/2	5.04	15,640
29 to RG	57 west of point 249	>-1/4	5.04	:3,63
30 to 20	32 east of point 230	15-1/2	4.91	1555
31 to 30		25-1/2	\$ <b>.</b> }3	13,030
31 to 80!	On a 38-in. are west of 231	15-1/2	5-35	إذة عرقة
31 to BE	-34	5-1/2	3.18	11,555
32 (2-4)	303 east of center, col 6	15-1/2	1.30	14,485
32 to 4A on		25-1/2	3.55	£, 45°
32 to 48 301	On a 44-in. are west of 232	15-1/2	3.75	ă - •
32 to 40 \ "		5-:1=	3.00	A 4 1
33 (2-4)	464 east of center, col 6	4)*1/E		
34 (2-4)	436 eact of certer, sol 6	10-1/2		
33 to 234			2.93	12,090
B <sup>L</sup> to M	96 was or policy 434	27-272	ڏ . ۽ ٽ	ి.చిన
34 to PB	97 east of point 34	5-1/4	ិ <b>រដ្</b> ច	;,.co
15t to sky co	e? cast of point all	.5-1/z	2.23	.2,195
34 to 48 301	24 cost of point 234	17-18-	عن - غ	12,440
84 to 20 1 d	23 east of joint 23k	5-1=	>	14,.75

Table 18

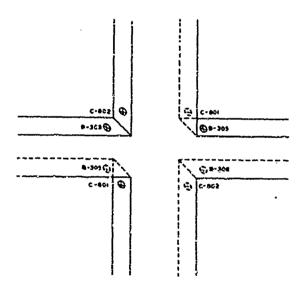
Results of Soniscope Tests

3d Floor Spandrel Beam at Column 8

Column-Beam Readings Through Outer 6 in.

North Side of Building

Station Numbers	Path Length	Diagonal Velocities fps
B-305	5.6	13,965
B-306	5-5	12,305
c-801	5.3	13,695
c-802	5.5	14,250

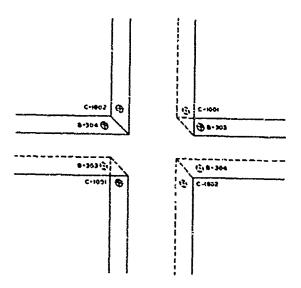


Station location diagram

Table 19

3d Floor Spandrel Beam at Column 10 Column-Beam Readings Through Outer 6 in. North Side of Building

Station Numbers	Path Lengthft	Diagonal Velocities fps
B-303	3.4	12,830
B-30 <sup>†</sup>	3.3	12,690
C-1001	3.6	14,575
C-1002	3.7	14,285



Station location liagram

Table ?^

3d Floor Spandrel Beam at Column 12 Column-Beam Readings Through Outer 6 in. North Side of Building

Station Numbers	Path Length ft	Diagonal Velocities fps
P-301	5.6	13,240
B-302	5.5	9,665
C-1201	5.3	13,250
C-1202	5.4	14,440

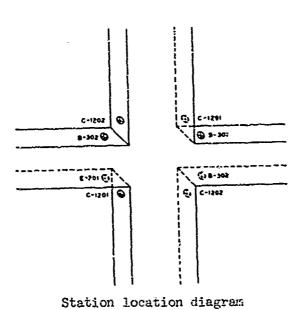
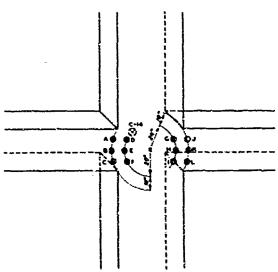


Table 21

3d Floor Spandrel Beam at Column 14

Readings Through Beams, Columns, and Column-Beam Connections (South Side)

-		Velocities îps
Station Numbers	Path Length	Straight Through Diagonals
C-14	1.17	13,765
C-14 to A	2.76	7,095
C-14 to B	2.76	10,000
C-14 to C	2.76	11,550
C-14 to D	2.03	11,215
C-14 to E	2.03	11,800
C-14 to F	2.03	11,665
C-14 to G	2.03	13,810
C-14 to H	2.03	13,810
C-14 to I	2.03	13,810
C-14 to J	2.76	13,730
C-14 to K	2.76	13,730
C-14 to L	2.76	13,730



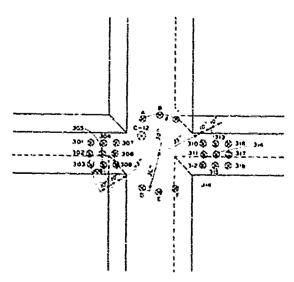
Station location diagram

Table 22

3d Floor Spandrel Beam at Column 12

Readings Through Beams, Column, and Column-Beam Connections (South Side)

Velocities				Dukk	Diamer?	
Station Numbers	Path Length <u>ft</u>	fp Straight Through	Diag- onals	Station Numbers	Path Length	Diagonal Velocities fps
C-12	1.17	14,445		C-12 to 306	÷.92	10,390
317	1.17	14,625		C-12 to 307	5.14	12,410
C-12 to A	2.76		19,500	C-12 to ·O3	3.14	12,410
C-12 to B	2.76		14,840	C-12 to 309	3.14	14,080
C-12 to C	2.76		14,605	C-12 to 310	: .14	12,.60
C C-12 to D	2.76		14,840	C-12 to 311	3.14	10,5%
C12 to 5	2.76		14,840	C-12 to 312	5 <b>.1</b> %	15,420
C-12 to F	2.76		14,840	C-12 to :13	92	11,665
C-12 to 301	5.13		12,835	C-12 to :14	65	11,430
C-12 to 302	5.13		10,470	C-12 to 1	5.92	11,770
C-12 to 30:	5.13		10,620	C-12 to 16	5.13	10,160
C-12 to 304	₹.92		8 <b>,6</b> 55	C-12 to :17	5.13	9 <b>,</b> £85
C-12 to 305	3.92		8,200	C-12 to 318	5.13	10,645

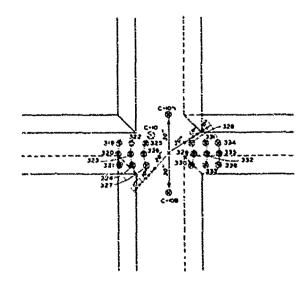


Station location diagram

Table 23
Results of Soniscope Tests

3d Floor Spandrel Beam at Column 10
Readings Through Beams, Column, and Column-Beam Connections
(South Side)
(Not Grouted)

	Path	Veloci fps			Path	Diagonal
Station Numbers	Length ft	Straight Through	Diag- onals	Station Numbers	Lengthft	Velocities fps
C-10	1.17	12,825		c-10 to 326	2,03	15,265
335	1.17	14,570		c-10 to 327	2.03	14,195
C-10 to C-10	A 2.76		14,525	C-10 to 328	2.03	10,355
C-10 to C-10	B 2.76		14,525	C-10 to 329	2.03	9,855
C-1C to 319	3.53		13,420	C-10 to 330	2.03	11,405
C-10 to 320	3.53		13,170	C-10 to 331	2.76	9,585
C-10 to 321	3.53		1h,175	C-10 to 332	2.76	10,495
C-10 to 322	2.76		14,300	C-10 to 333	2.76	3 <b>,</b> 680
C-10 to 323	2.76		15,080	C-1.0 to 334	3.53	9,050
C-10 to 324	2.76		14,225	C-10 to 335	3.53	8,715
C-10 to 325	2.03		16,370	C-10 to 336	3.53	10,170



Station location diagram

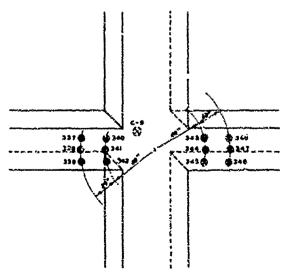
Table 24

#### 3d Floor Spandrel Beam at Column 8

Readings Through Beams, Columns, and Column-Beam Connections (South Side)

(Not Grouted)

		Veloc	
Station Numbers	Path Length ft	Straight Through	Diagon.
c-8	1.1.7	15,195	
c-8 to 337	5.13		10,020
C-8 to 338	5.13		13,035
C-8 to 339	5.13		11,305
C-8 to 340	3.53		13,320
C-8 to 341	3-53		12,430
C-8 to 342	3.53		10,895
c-8 to 343	3.53		14,350
C-8 to 344	3-53		14,350
C-8 to 345	3.53		14,350
C-8 to 346	5.13		14,210
c-8 to 347	5.1.3		13,902
C-8 to 348	5-13		14,100



Station location diagram

TO FOR A POSSION OF A POSSION O

Table 25

### Results of Schlacope Tests

3d Floor Spandrel Pean at Column 6

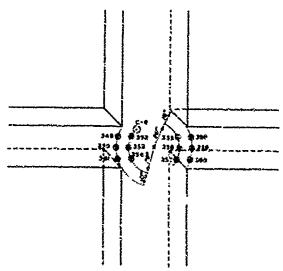
Readings Through Beams, Columns, and Column-Beam Connections

(South Side)

(Grouted)

	The state of the s	Velocitles îps
Station Numbers	Peth Length	Straight Through Diagonals
c-6	1.17	14,810
C-6 to 349	2.76	12,105
C-6 to 350	2.76	.3,940
C-6 to 351	2.76	,, )
C-6 to 352	2.03	;
C-6 to 353	2.03	ð
C-6 to 354	2.03	,500
C-6 to 355	2.03	12,455
C-6 to 356	2.03	14,400
c-6 to 357	2.03	13,270
C-6 to 358	2.76	13,205
C-6 to 359	2.76	13,305
C-6 to 360	2.76	12,050

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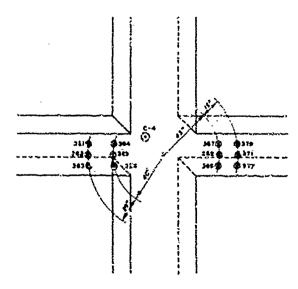
Station location diagram

Table 26

3d Floor Spandrel Beam at Co. umn 4
Readings Through Beams, Columns, and Column-Beam Connections
(Couth Side)

(Grouted)

		Velocities
Station Rumbers	Pach Leagth <u>ft</u>	fps Straight Through Diagonals
C-4	1.17	14,445
C-4 to 361	5.13	9,625
C-4 to 362	5.13	10,200
C-4 to 363	5.13	9,450
C-4 to 364	3.53	12,930
C-4 to 365	3.53	10,600
C-4 to 366	3-53	12,255
C-4 to 367	3-93	11,360
C-4 to 368	3.93	12,360
C-4 to 369	3.93	11,800
C-4 to 370	5.13	8,170
0-11 to 371	5-13	11,450
C-4 to 372	5-13	9,810

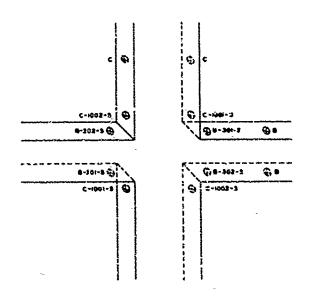


Station location diagram

Table 27

3d Floor Spandrel Beam at Column 10 Column-Beam Readings Through Outer 6 in. South Side of Building

And the second s		Velocities 1°ps			
Station Numbers	Path Length	Streight Through Diagonals			
B-301-S	3.7	13,355			
B-302-8	3.7	13,755			
C-1001-S	<b>3.</b> 8	13,620			
C-1002-S	3.75	13,890			
C	1.7	15,180			
B	2.6	14,130			

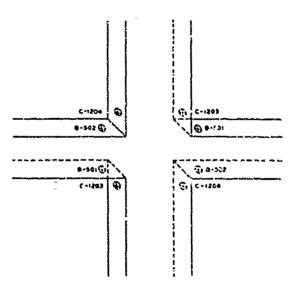


Station location diagram

Table 28

## 5th Floor Spandrel Beam at Column 12 Column-Beam Readings Through Outer 6 in. North Side of Building

Station Numbers	Path Lengthft	Diagonal Velocities fps
B-501	5:6	12,785
B-502	5.7	10,775
C-1203	5.4	14,675
C-1204	5.3	14,135

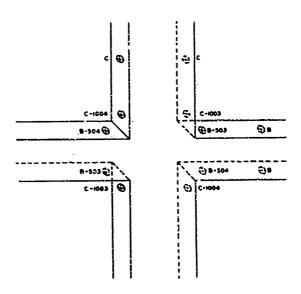


Station location Hiagram

Table 29

## 5th Floor Spandrel Beam at Column 10 Column-Beam Readings Through Outer 6 in. North Side of Building

		Velocities fps		
Station Numbers	Path Length  ft	Straight Through Diagonals		
B-503	3.3	14,165		
B-504	3.6	14,285		
C-1003	3.6	15,126		
C-1004	3.9	14,285		
С	4.3	15,560		
В	2.6	15,030		



Station location diagram

Table 30

Results of Scniscope Tests

East Stairway, Wall 2 Between Wall T and Fan Rocm Wall

First Floor

(Not Grouted)

		eferences	<del></del>	<del></del>		*
Station Numbers	Distance from Fan Room Wall in.	Distance up from Floor in.	Path Length ft	Straight	ខ្លួន	Remarks
57-161	14	136	1.00	15,875		
SW-102	42	136			-	Only one side ac-cessible
SW-103	85	136				Only one side ac-cessible
SW-104	122	136				Only one side ac-cessible
S#-105	162	136				Only one side ac-cessible
SW-106	192	136				Only one side ac-cessible
ew-1.07	162	84				Only one side ac-cessible
S#-108	120	64				Only one side ac-cessible
5W-109	62	84				Only one side ac-
102 to 109			4.42		8,110	
103 to 109			4-35		8,880	
103 to 108	-		4.81		7,990	
104 to 108			3.98		13,050	
104 to 107			4.99		9,325	-
10ó to 107			4.31		8,850	

Table 31

Results of Soniscope Tests

East Stairway, Wall 2 Retween Wall T and Fan Room Wall

Second Floor

	Station R	eferences		<del></del>	* <del></del>	
	from Fan Room Wall	Distance up from Floor	Path Length	Straight	ps	
Station Numbers	in.	in.	<u>ft</u>	Through	Diagonals	Remarks
SW-201	14	139	1.00	14,705		
SW-202	42	139	1.00	15,875		
SW-203	82	139	1.00	15,875		-
sw-204	122	139				Only one side ac-cessible
SW-205	162	139				Only one side ac-
ଞ୍ଜ - 206	192	139				Only one side ac- cessible
sw-207	177	102				Only one side ac-cessible
₋sw-20ô	122	88				Only one side ac-
SW-209	66	72				Only one side ac-
201 to 202			2.52		11,405	
202 to 209			5.77		8,730	
203 to 209			5.38		10,530	
203 to 208			5.25		10 <b>,</b> 690	
205 to 208			5.20		10,505	
204 to 207			5.43		14,440	
206 to 207		-	3-54		13,615	

Table 32

Results of Soniscope Tests

East Stairway, Wall 2 retween Wall T and Fan Room Wall

Third Floor

	Station E	eferences				
	Distance	ererences				
	from	Distance		Veloci	ties	
-	Fan Room	up from	Path	fy	ာန	
Maddin Wallana	Wall	Floor	Length	Straight	D2 = 2 =	D
Station Numbers	in.	in.	_ft_		Liagonals	Remarks
SW-301	28	120	1.00	15,875		
SW-302	68	120				Only one side ac-cessible
sn-303	108	120	÷			Cnly one side ac- cessible
SW-304	146	120		-		Only one side accessible
SW-305	188	120				Only one side ac-cessible
sw-306	158	85				Only one side ac-cessible
SW-307	108	60				Only one side ac-cessible
sw-308	68	70				Only one side ac-cessible
301 to 302			3.48	-	9,945	-
301 to 308		-	5.62		4,030	
303 to 308			5.30		13,185	
302 to 307			5.92		11,065	
304 to 307			6.17	-	12,390	
303 c 306			5.23		12,395	
305 to 306			3.81		13,705	-

Table 33

Results of Soniscope Tests

East Stairway, Wall 2 Between Wall T and Fan Room Wall

Fourth Floor

(Not Grouted)

	Station R Distance	eferences				
-	from Fau Room Wall	Distance up from Floor	Path Length		íties ps	
Station Numbers	in.	in.	<u>ft</u>	Through	Diagonals	Remarks
SW-401	1.4	125	1.00	14,925		
SW-402	41	145	1.00	15,625	-	
sw-403	81	125				Only one side accessible
SN-404	121	125				Only one side accessible
sw-405	161	125				Only one side accessible
sw-406	211	125				Only one side ac-cessible
SW-407	161	95				Only one side ac-cessible
sw-408	121	71			Ē	Only one side ac-cessible
2M-jr03	70	65				Only one side ac- cessible
401 to 402			2.62		10,040	
402 to 409			5.51		6,090	
403 to 409			5.08		12,180	
403 to 408			5.65		12,470	
404 to 468			4.83		12,675	
404 to 407			3.98		12,715	
406 to 407			14 • Off		12,705	
405 to 408			6.06		12,120	

Table 34 <u>Assists of Somiscope Tests</u> Mechanical Exifernt Room Packah, Wall 13

		Meglanical Exipment Ro	000 B-254A, ¥6			
		eferences Distance up from	Pata Langua	Veloci Straignt	tie", fpa	
Statica Rumbers	Distance from Morth. Face Wall D, in.	Concrete Transcold, in.	fit Dength		Distriction.	Feet.
101	39	50	0.67	12,307		
103	e9 -	115				Unly the size un essible
ιυ <u>3</u>	ئے۔	.00				(fil) war ziot bereabille
164	151	<u>.</u> •				Ctly one side decessitua
\$ <b>0</b> 5	109	1.				Only one side eccessible
100	35	5÷	3.57	14,500		
10;		7	,	- ,,,		Only case side accessible
-0#	Ł	#* <b>6</b> 5				this use also arcessite
109	- -	73				(thy one side acressible
110	1					Chip one of the necessible
		3:				•
111	<u>.</u>	**				Only one lide accessible
112	40	14.1 				Cally one place accessible
113	3Ê	)				Coly one pide acressible
114	50	<i>y</i> ,	0.63	15,510		
215	69	Ħ	5.67	15,000		
110	15	୍ଦୃତ୍ତ				Coly one with accessible
11 -	55	453				Galy est bide thresait/e
118	73	150				Unity our side accessible
11)	1	192	-			tally one side occrasible
	Distance up them South Corner of Door	Distance South				
12 <del>ĉ</del>	23		U.c.;	15,100		
		(درته				
129	29 *-	.45.5 .5	J.5;	<u>ئۇد</u> رىد		Unly the life threshifte
130	34	6.3Û				•
.31	€0	- A - T				(miy one olde necessible
135	12	255				(higher olde secessial)
133	¢	£				Unit one tile sometative
15-	<u>5-</u>	<u>\$</u>				Only one alter secressible
135	<u>i</u> ū	, a.,				(mly one plue accessable
******	•35	1.7.,				Cally on side becomesible
124	•35	ing of				Carr can side accessione
151 to 132			*.*		1,512	
13, to 103			5 <u>-</u> -		-,3,7	
131 to 10-			₩•5~		ترخيز وتسم	
101 🌝 165	-		3.09		ناحق و هذ	
"Ot to 13°			<b>≈</b> •3 <sup>™</sup>		45:- 5	
Lib to .3:			-44 <sup>±</sup>		ر"در <del>ه و</del> سده	
100 to 139			* **		ٽٽ <b>ج</b> و شو	
.00 to6			and the		1300 7	
.30 20;		-	5•==		يمروعة	
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224 to 125			4.1.		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
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lai to 13s			. ***		e a grand	
127 % L34					بيتجافو عب	
					e ± 9 ° €0.	
125 % 135						
163 to 1834			• -			
.29 ≤ .35			1 <b>4</b>		***	
129年37			• *		*** **	
46) W 15			•- •		-94-2Î	

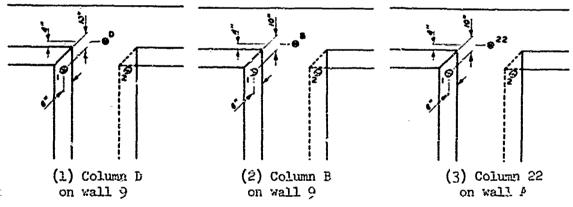
Table 35
Results of Soniscope Tests

## Readings on Column-Beam Connections

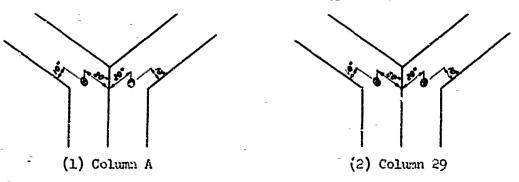
# Around Outpatient Clinic

(Grouted)

14,935 15,130
15,130
2,9230
12,555
12,555
15,325
15,325
11,455
14,305



A. Column-beam connections (grouted)



B. Corner columns (grouted)

Table 36

Velocity Tests of Epoxy-Repaired Cores

Test Condition	Velocities, fps		
	Film	1/16-in. Layer	1/8-in. Layer
Saturated, before breaking	14,645	14,540	14,645
After 24 hr curing at 70 to 80 F	14,410	11,060	14,000
After 24 hr curing at 150 F	13,575	13,520	13,730
After 43 hr curing at 120 F	13,710	13,650	13,730
Saturated, final reading	13,575	14,060	13,730